Codification and load combination factors

Appropriate generalisation of (environmental) time variant loads and their combination

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Motivation

• Partial factor formats are highly simplified/generalized.

$$\Sigma F_{d} = \sum_{i} \gamma_{G,i} G_{k,i} + \gamma_{Q,l} Q_{k,l} + \sum_{j>l} \gamma_{Q,j} \psi_{0,j} Q_{k,j} + (\gamma_{P} P_{k})$$
or
$$\Sigma F_{d} = \begin{cases} \sum_{i} \gamma_{G,i} G_{k,i} + \gamma_{Q,l} \psi_{0,l} Q_{k,l} + \sum_{j>l} \gamma_{Q,j} \psi_{0,j} Q_{k,j} + (\gamma_{P} P_{k}) \\ \sum_{i} \xi_{i} \gamma_{G,i} G_{k,i} + \gamma_{Q,l} Q_{k,l} + \sum_{j>l} \gamma_{Q,j} \psi_{0,j} Q_{k,j} + (\gamma_{P} P_{k}) \end{cases}$$
or
$$\Sigma F_{d} = \begin{cases} \sum_{i} \gamma_{G,i} G_{k,i} + (\gamma_{P} P_{k}) \\ \sum_{i} \xi_{i} \gamma_{G,i} G_{k,i} + (\gamma_{Q,l} Q_{k,l} + \sum_{j>l} \gamma_{Q,j} \psi_{0,j} Q_{k,j} + (\gamma_{P} P_{k}) \end{cases}$$
(8.13)
$$\end{cases}$$
(8.14)

Motivation

• The revision of the Eurocodes is an opportunity to revisit code calibration.





Motivation

• The tentative calibration results are based on rather genric representation of variable loads.

| Table 3: Load Variables | | | | | |
|--------------------------------|--------------|------------|--------|------------|----------|
| Variable | Distribution | Mean Value | C.o.V. | char.value | fractile |
| Load Effect MU Frames | lognormal | 1.000 | 0.100 | 1.000 | not used |
| Self weight steel | normal | 1.000 | 0.025 | 1.000 | not used |
| Self weight concrete | normal | 1.000 | 0.050 | 0.980 | not used |
| Self weight glulam | normal | 1.000 | 0.100 | 0.950 | not used |
| Self weight timber | normal | 1.000 | 0.100 | 0.950 | not used |
| Self weight masonry | normal | 1.000 | 0.070 | 1.000 | 0.500 |
| Self weight aluminum | normal | 1.000 | 0.040 | 1.000 | 0.500 |
| Self weight soil | normal | 1.000 | 0.050 | 1.000 | 0.500 |
| Permanent load small V | normal | 1.000 | 0.100 | 1.000 | 0.500 |
| Perm <u>anent load large V</u> | normal | 1.000 | 0.200 | 1.329 | 0.950 |
| Wind MU | lognormal | 0.970 | 0.260 | - | - |
| Wind 50a-extreme | gumbel | 1.000 | 0.140 | 1.084 | 0.980 |
| Snow MU | lognormal | 0.810 | 0.260 | - | - |
| Snow 50a-extreme | gumbel | 1.000 | 0.200 | 0.821 | 0.980 |
| Imposed MU | lognormal | 1.000 | 0.100 | - | - |
| Imposed 50a-extreme | gumbel | 1.000 | 0.260 | 1.350 | 0.990 |
| | | | | | |

Challanges

- Environmental loads like wind and snow are represented as 98% fractile of the corresponding yearly extreme value distribution.
- Evidence from data is not very consistent.
- Spatial variability of magnitudes is considered by "zones".
- Spatial variability of coefficient of variation is ignored.
- The effects of climate change are ignored.

Example: Wind, Norway

- Assessment of weather station data.
- Assuming stationarity.
- Variability of CoV of v_{max}^2 0.24. 0.31.
- Represent v_{max}^2 with Gumbel seems ok.



Example: Snow, Norway

- Simulated snow (from data on precipitation and temperature).
- Only preliminary assessment of data.
- Variability of CoV of $s_{0,max}$ 0.4 0.7.



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Figure 17: Percentile comparison of SWE for Lom, Innlandet with Eurocodes and Klima2050 study.

Example: Snow, Norway

- Simulated snow (from data on precipitation and temperature).
- Only preliminary assessment of dat
- Variability of CoV of $s_{0,max}$ 0.4 0.7.
- Large discripancy to current characteristic values.
- Rather evident non-stationarity.



Example: Wind, Denmark

- Simulations based on different climate change scenarios.
- Change in characteristic value +/- 10 %.
- CoV keeps similar.



(b) sfcWindmax, med ekstremkurver

Example: Snow, Denmark

- Simulations based on different climate change scenarios.
- Decrease in characteristic approximately 30%
- Increase in COV



Approach for calibrating load combination factors.

- 1. Represent design equations with 1 variable load and calibrate γ_G , $\gamma_{Q,snow}$, $\gamma_{Q,wind}$, $\gamma_{Q,imposed}$.
- 2. Represent design equations with 2 variable loads, keep $\gamma_G, \gamma_{Q,snow}, \gamma_{Q,wind}, \gamma_{Q,imposed}$ fixed and calibrate $\psi_{0,i}$.
- Reliability analysis for step 2 necesitates the solution of the load combination problem.

Load Combination

Load Combination Factors:

- Load combination factors (Ψ_0, Ψ_1, Ψ_2) are essential in determining design values for ultimate and serviceability limit states.
- EN 1990 (Annex C) establishes these factors based on Ferry-Borges-Castanheta's (FBC) simplified load combinations.
- The factors depend on:
 - Coefficients of variation of annual maximum loads.
 - Frequencies of the loads.
 - Duration of the extreme loads.
 - The likelihood of loads occurring simultaneously, which can be modelled using conditional distribution functions.



Load Combination

Impact of Climate Change on Load Combinations:

- Climate change may alter
 - The magnitudes of annual maximum loads.
 - the coefficients of variation for annual maximum loads.
- Other weather phenomena not covered by present codes may become important, e.g. for wind actions.
- Increased frequencies and duration of combined loads in the FBC model are likely due to changing climatic conditions.
- These changes could necessitate adjustments to load combination factors (Ψ_0).
- Load duration factors (Ψ_1 and Ψ_2) could also be affected by altered frequencies of extreme loads.

Load Combination

Data Analysis Challenges:

- No specific analyses on load combinations have been conducted due to:
 - Insufficient data availability.
 - High variability and inhomogeneity in the existing data.

Takeaway:

• Further studies are needed to assess the impact of climateinduced changes on load combination factors and their implications for structural design standards.