Non-Stationary Structural Reliability Analysis in the Changing Climate

Professor Mahesh Pandey

Dept. of Civil and Environmental Engineering University of Waterloo Waterloo, CANADA

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Structural Safety in the Changing Climate

- A rapid pace of climate change is evident by increasing frequency and intensity of weather extremes
 - Record breaking heat waves, wildfires, rain/snow storms, and hurricanes in many parts of the world.
- Increasing severity of weather extremes is threatening the safety and functionality of existing infrastructure systems
 - Transportation systems, electrical networks, and water infrastructure suffer damaged and disrupted
- Design codes and standards must adapt to changing climate to maintain a high level of safety

Design in the Changing Climate

New models and methods are needed to account for climate change effects in the design of infrastructure systems

General Idea

- A structure is expected to face a sequence of loads of uncertain magnitude, arriving randomly over the service life, (0, *t*)
- The structure must withstand all such loads while ensuring the safety of occupants (many limit states of performance)
- A sequence of uncertain load events can be naturally modelled as a stochastic Point Process

A Practical Approach

- The <u>time-variant</u> stochastic analysis can be replaced by a <u>time-invariant</u> analysis
- The concept of "extreme value distribution" plays a key role
- A structure is safe over its lifetime, if it can withstand the maximum of all load events that could occur over its lifetime

Key Elements

A structure of uncertain strength, R, is exposed to a stochastic load process over a time interval, (s, t],

- The distribution of maximum load, *X_{max}*(*s*, *t*), generated by the process over the interval , (*s*, *t*]
- Computation of the probability of failure, $P_f(s, t)$

$$P_f(s,t) = \mathbb{P}\left[R - X_{max}(s,t) \le 0\right] \tag{1}$$

- How to derive the distribution of maximum load under non-stationary conditions?
- Other items of interest
 - Calibration of load and resistance factors
 - Target reliability considerations
 - Degradation of structural strength

Shock Process (Marked Point Process)



- Random components of the model: (T, X) vectors of RV
 - Inter-arrival times, *T*₁, *T*₂,...
 - Load magnitudes, X₁, X₂,...
 - Time-dependent frequency (rate)
 - Time-dependent loads

• "Return period" is a widely used term in design codes

(1) Inter-Arrival Time



- Average time between two consecutive events
- There are multiple return periods: $\mathbb{E}[T_1], \mathbb{E}[T_2], \dots, \mathbb{E}[T_n]$
- (2) Waiting Time to Next Event

$$\begin{array}{c|c} & \bullet & \bullet \\ 0 & S_1 & S_2 & S_n & t & S_{n+1} \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & & \\ & & & & & \\ & & & & & & \\ & & & & & & \\ & & & &$$

• Mean waiting time to next event at time $t = \mathbb{E}[W(t)]$

Stationary Climate: Basis of Current Design

• Current design codes are based on the stationary climate conditions

Homogeneous Poisson Process (HPP)

- A common model of climate loads in structural reliability
- The load occurrence rate, a constant (λ)
- Inter-arrival times, T1, T2, ..., IID exponential RVs
- Load magnitudes, X₁, X₂,..., IID with a common DF

Return Period in Stationary Climate

- Average inter-arrival times are all EQUAL, $\mathbb{E}[T_n] = \frac{1}{\lambda}, \forall n > 0$
- $\bullet\,$ There is a SINGLE return period \rightarrow convenient in design
- Extreme Load Distribution

$$\mathbb{P}\left[X_{max}(t) \leq x\right] = e^{-\lambda t(1 - F_X(x))}$$

It depends on the length of the interval only (owing to stationary process)

Stationary Reliability Analysis

Annual extreme load distribution from HPP model

$$\mathbb{P}\left[X_{Amax} \leq x
ight] = e^{-\lambda(1-F_X(x))}$$

- Asymptotic extreme value distributions are also used (Gumbel distribution)
- Annual probability of failure is the same for every year in the service life

$$P_{fA} = \mathbb{P}\left[R - X_{Amax} \leq 0
ight]$$

Annual probability of failure is kept below a target level (by code calibration)

Time-Invariance

All reliability measures are time-invariant in the stationary climate

- The climate change is causing temporal variations in the frequency and intensity of weather extremes
- Sustained global warming over a long period of time is likely to introduce some dependence among weather extremes
- Increasing concentration of greenhouse gases with time will continue to amplify non-stationary effects
- Non-stationary processes are required to model such temporal changes in the climate variables

Key Points

- Stationary load processes will not be valid in the changing climate
- All reliability measures will become time-variant quantities

- Non-stationary reliability analysis to model climate change effects
- Scope of research
- Analytical developments in stochastic analysis
- Statistical estimation of model parameters
- Numerical parametric study
- Practical data analysis and modelling
- Design code development issues

Non-Stationary Point Processes

Load Arrivals

Non-Homogeneous Poisson process (NHPP)

- A natural extension of the homogeneous Poisson process
- The occurrence rate is time dependent, $\lambda(t)$
- Arrival times (T₁, T₂,...) are dependent RVs
- "Independence" property still holds
 - Number of events in an interval are independent of any other disjoint interval
 - This property is also a limitation of the model

In Non-Homogeneous Birth Process

- Includes all NHPP properties, but "Independence" property relaxed
- Number of events in an interval depends on the history of the process

Load Magnitudes

A function of the time of load arrival

$$X(\mathbf{s}_k) = \phi_1(\mathbf{s}_k) + \phi_2(\mathbf{s}_k)X_k$$

• $\phi_1(s_k), \phi_2(s_k)$ are time-dependent amplification functions

Non-Homogeneous Poisson Process

• Fully analytically tractable model

The probability distribution of the number of events, N(s, t), s < t,

$$f_N(k; \boldsymbol{s}, t) = \frac{(\Lambda(\boldsymbol{s}, t))^k}{k!} \boldsymbol{e}^{-\Lambda(\boldsymbol{s}, t)}, \quad (0 \le k < \infty)$$
(2)

- NHPP is defined by the Mean Value Function, $\Lambda(s, t) = \mathbb{E}[N(s, t)]$.
- The occurrence rate (or frequency), $\lambda(t) = rac{\mathrm{d}\Lambda(t)}{\mathrm{d}t}$
- If the rate is increasing, inter-arrival times periods will be decreasing, and E [*T_n*] < · · · < E [*T₂*] < E [*T₁*]
- Analytical results were derived for all necessary quantities required for structural reliability analysis
 - Return period, mean waiting time
 - Extreme value distribution with time-dependent loads

Pandey, M.D., and Lounis, Z. (2023). Stochastic modelling of non-stationary environmental loads for reliability analysis under the changing climate. Structural Safety, 103, 102348, pp.1-11.

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• Distribution of inter-arrival time, *T_n* (complementary CDF)

$$\overline{F}_{T_n}(t) = \int_0^\infty f_{T_1}(t+u) \frac{[\Lambda(u)]^{n-1}}{(n-1)!} \,\mathrm{d}u$$

• The mean inter-arrival time or *n*th return period

$$\mathbb{E}\left[T_{n}\right] = \int_{0}^{\infty} \overline{F}_{T_{1}}(s) \frac{\left[\Lambda(s)\right]^{n-1}}{(n-1)!} \,\mathrm{d}s, \quad (n \geq 1)$$

• The return period is no longer a constant, rather it changes with n

Non-Stationary Reliability Analysis

• The extreme load distribution in a time interval, $(t_1, t_2], t_1 < t_2$:

$$F_{max}(x, t_1, t_2) = e^{-\Lambda(x, t_1, t_2)}$$

where the mean value function is given as,

$$\Lambda(x, t_1, t_2) = \int_{t_1}^{t_2} \overline{F}_X(\psi(x, s)) \lambda(s) \mathrm{d}s, \quad \text{and} \quad \psi(x, s) = \frac{x - \phi_1(s)}{\phi_2(s)}$$

Probability of failure in a time interval

$$P_f(t_1, t_2) = \mathbb{P}\left[R - X_{max}(t_1, t_2) \leq 0
ight]$$

Computation using FORM or simulations

Non-Stationary Climate

- Time-invariance property of all the reliability measures is lost
- All the measures must be defined with reference to a time interval

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Conditional Intensity

Conditional probability of an event given the history of the process, $\lambda_C(n, t)$

$$\mathbb{P}\left[N(t+\mathrm{d}t)-N(t)=1|N(t)=n\right]=\lambda_{\mathcal{C}}(n,t)\mathrm{d}t+o(\mathrm{d}t),$$

- Form of intensity function, $\lambda_C(n, t) = h(n)\lambda(t)$
- Includes dependence on the number of past events (n)
- Also depends on the time (t) elapsed since the start of the process
- Various types of birth processes depending on the cond. intensity
 - Homogeneous form, $\lambda(t) = \lambda$
 - Yule process, Generalized Polya process, ...
 - NHPP is a special case, $\lambda_C(n, t) = \lambda(t)$
- Analytical solutions
 - Homogeneous birth process all analytical solutions have been derived
 - Non-homogeneous case analyzed for a linear form of intensity function

Linear Extension of the Yule Process (LEYP)

Conditional intensity

$$\lambda_{C}(n,t) = \underbrace{(an+b)}_{\textit{past history}} imes \underbrace{\lambda(t)}_{\textit{time elapsed}}, \quad (a \ge 0, b > 0)$$

- History dependence: A linear form, h(n) = (an + b)
- Temporal effect: A time-dependent rate, λ(t)
- Parameter, *b* = 1, fixed to ensure the model identification
- Scale parameter, a, controls the influence of the process history

• Negative binomial distribution for the number of events

$$\mathbb{P}\left[N_{t}=n\right]=\frac{\Gamma\left(\alpha+n\right)}{\Gamma(n+1)\Gamma\left(\alpha\right)}\left(\beta_{t}\right)^{\alpha}\left(1-\beta_{t}\right)^{n}$$

Pandey, M.D., and Mercier, S. (2024). Stochastic Modelling of Non-Stationary and Dependent Weather Extremes for Structural Reliability Analysis in the Changing Climate. Structural Safety (under review)

General Non-Stationary Load Model

LEYP Shock Process



Non-stationary components

- Frequency of occurrence of load events
- Dependence on the number of events occurring over time
- Intensity (magnitude) of loads are also time-dependent

Analysis Results

- Expressions for any nth return period, mean waiting time to the next event
- Correlation coefficient between the number of events in two intervals (dependence measure)
- Distribution of maximum loads in (*s*, *t*]

Reliability Analysis

- Time-dependent loads, $X(s_k) = \phi_1(s_k) + \phi_2(s_k)X_k$
- Distribution of maximum load in a given time interval

$$F_{max}(x, t_1, t_2) = \left[1 + \frac{\mu_{12}}{\alpha} - aq^*(x, t_1, t_2)\right]^{-\alpha}$$
$$q^*(x, t_1, t_2) = \int_{t_1}^{t_2} \frac{\lambda(s)}{\beta_s} F_X(\psi(x, s)) ds$$

with
$$\psi(x,s) = \frac{x - \phi_1(s)}{\phi_2(s)}$$
, $\beta_s = e^{-a\Lambda(s)}$, and $\Lambda(s) = \int_0^s \lambda(u) du$

Probability of failure in a given time interval

$$P_f(t_1, t_2) = \mathbb{P}\left[R - X_{max}(t_1, t_2) \le 0\right] = \int_0^\infty \overline{F}_{max}(x, t_1, t_2) f_R(x) \mathrm{d}x$$

where

Inter-Arrival Times

*n*th return period, *T_n* = *S_n* - *S_{n-1}*, *n* > 1, *S*₀ = 0
 CCDF of *T_n*:

$$\overline{F}_{T_n}(u) = \frac{a}{B\left(\frac{b}{a}, n-1\right)} \times \int_0^\infty \lambda\left(s\right) e^{-g(u,s)} \left(1 - e^{-a\Lambda(s)}\right)^{n-2} ds$$

where

$$g(u,s) = a(n-1)\Lambda(s,u+s) + b\Lambda(u+s)$$

• An *n*th return period by integration: $\mathbb{E}[T_n] = \int_0^\infty \overline{F}_{T_n}(u) du$

Variable Return Periods!

In a non-stationary process, $\mathbb{E}[T_1], \mathbb{E}[T_2], \dots, \mathbb{E}[T_n]$, are all DISTINCT values

Correlation coefficient

Between the number of events in two adjacent intervals, $(t_1, t_2]$ and $(t_2, t_3]$ with $0 \le t_1 < t_2 < t_3$

Definition

$$\rho[N_{12}, N_{23}] = \frac{\text{COV}[N_{12}, N_{23}]}{\sigma_{12}\sigma_{23}}$$

• A general expression

$$\left[\rho\left(\mathsf{N}_{12},\mathsf{N}_{23}\right)\right]^{2} = \left(\frac{\mu_{12}}{\alpha + \mu_{12}}\right) \left(\frac{\mu_{23}}{\alpha + \mu_{23}}\right)$$



Linear Rate Function

- The reference period for the analysis is 2020 - 2100, (*t_e* = 80 years)
- The base case is the stationary climate with a rate, λ_o = 1 event/year
- An overall, linear increase in the occurrence rate is kλ_o over t_e years

Climate Amplification Factors

Increase as a multiple of the base case (stationary climate) Increase in the frequency: $k = k_F$ Increase in the load magnitude: k_L

Parametric Study

- Investigation of the effect of various model parameters
 - Correlation coefficient (dependence)
 - Expected number of events
 - Return periods
 - Mean waiting time
 - Probability of Failure
- Model parameters
 - Scale parameter of the history function
 - Base rate
 - · Liner increase in the rate and load magnitude
 - Amplification factors for the rate and the load

Parameter	Notation	Values
Scale	а	0.1 - 1
Base rate	λ_0	0.02 - 1
Amplification Factors	-	-
Frequency (rate)	k _F	1 - 4
Load	k _L	1 - 1.4

Dependence in LEYP



- The decadal correlation coefficient between the number of events (t ± 10)
- Results for homogeneous birth process
- A constant rate, $\lambda = 0.02$ /year

- Corre. coeff. increases continuously as time increases from 10 80 years
- The rate and the magnitude of this increase are controlled by "a"

Example: Inter-Arrival Times



- The first 6 return periods (RPs) are plotted
- A dramatic decrease in subsequent RPs of extreme events
- Frequency amplification and the scale (a) parameter have significant influence on the inter-arrival times

Mean Waiting Time to the Next Event



- Mean waiting time decreases with the passage of time
- Results are shown for a = 0.5
- Mean waiting time decreases with an increase in the frequency amplification

- The mean waiting time (MWT) is a more useful measure than mean inter-arrival time
- MWT does not require any information about the history of the process

Probability of Failure: Increasing Frequency



- Cumulative probability of failure in P_f(0, t)
- Scale, *a* = 0.5, λ₀ = 0.1
- 50 to 100% increase in frequency over 50 years
- Time-invariant load

- The impact of frequency amplification on the probability of failure
- An order of increase in k_F has a modest effect on $P_f(0, t)$

Probability of Failure: Increasing Intensity



- Cumulative probability of failure in P_f(0, t)
- Scale, *a* = 0.5, λ₀ = 0.1
- Frequency amplification,
 k_F = 1.5, fixed
- load intensity increase 20 to 40% over 50 years

- The impact of load amplification on the probability of failure
- A modest increase in k_L has a significant effect on P_f(0, t)

Application: Extreme Precipitation in Future



- Future climate forecast by the Canadian Earth System Model (5.03)
- Simulation data for heavy precipitation events (> 35 mm/day)
- Scenario: SSP 5-8.5 (Fossil-fuelled Development)
- Expected increase of 4.4°C in the the mean global temperature by 2100
- Data for a spatial grid-box of of $6 \times 10 \ km^2$ size in Toronto, Ontario

Precipitation Data: LEYP Model



- Expected number of events in (0, *t*)
- Base rate λ₀ = 0.91
- Frequency amplification, $k_F = 1.55$
- Intensity amplification is absent, $k_L = 1$

- Three stochastic effects are present
 - **(1)** A stationary rate, $\lambda_o = 0.91$, implies 73 events expected in 80 years
 - Frequency amplification by 55% over 80 years increases to 92 events
 - Dependence effect increasing this to 132 events
- Modest dependence, decadal corre. coeff. increases from 0.07 to 0.17
- In spite of this, dependence has a discernible effect

- Non-stationary stochastic load models are developed for structural reliability analysis in the changing climate
 - NHPP and LEYP models are analysed in detail
 - The "order statistics property" is a key to derive analytical results
- Analytical results are derived for various elements of reliability analysis framework
- The LEYP model overcomes a major limitation of the classical Poisson process by including the statistical dependence among extreme events
 - Even a mild dependence can lead to a significant increase in the frequency of extremes and the probability of failure

Path forward

- Code calibration in non-stationary climate
- Target reliability considerations
- Degradation of structural strength
- Stochastic load combination rules

Thank you for your attention!

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Non-Stationary Modelling: Current Approach

- In the literature, very limited use of non-stationary stochastic processes for modelling the climate change effects
- The Gumbel (or GEV) distribution with time dependent parameters is the state-of-the-art
- Parameter estimation using annual maxima data obtained through climate simulation models
- Using this, the annual probability of failure is computed
- The annual maxima distribution (and probability of failure) in a given year is assumed to be "Independent" of all other years
- This "Independence" implies that extremes are generated by an NHPP
- The current approach is quite restrictive
 - It does not allow to investigate other aspects of the problem (inter-arrival times, dependence, intensity and frequency amplification)